

ICC-ES Evaluation Report

ESR-6004

Issued September 2026

Revised October 2026

This report remains under an active [continuous compliance program](#) through the end of September 2027.

This report also contains:

- [City of LA Supplement](#)

- [CA Supplement](#)

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DIVISION: 06 00 00— WOOD, PLASTICS AND COMPOSITES Section: 06 02 00— Design Information Section: 06 18 13 – Glued-Laminated Beams	REPORT HOLDER: BOISE CASCADE WOOD PRODUCTS, LLC	EVALUATION SUBJECT: KING BEAM 3000 AND BOISE GLULAM	
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1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2024, 2021, 2018 and 2015 [International Building Code® \(IBC\)](#)
- 2024, 2021, 2018 and 2015 [International Residential Code® \(IRC\)](#)

Property evaluated:

- Structural

2.0 USES

KING BEAM® 3000 and BOISE GLULAM® are used as structural components, such as beams, headers, rafters, purlins and columns, in mass timber buildings where permitted by the building codes.

3.0 DESCRIPTION

3.1 General:

3.1.1 KING BEAM 3000: KING BEAM is glulam beam and has two grades, 30F-E4 and 30F-E5. KING BEAM 3000 is manufactured with a proprietary balanced and an unbalanced layup combination, respectively, and documented in the approved in-plant manufacturing standard. KING BEAM 3000 30F-E4 glulam beams use conventional laminating lumber grades of Douglas fir-Larch in the tension and compression zones, and Southern pine in the core with the exception that the outermost tension and outermost compression laminations are made of a proprietary Douglas fir-Larch manufactured lumber tension lamination, as permitted by ANSI A190.1. KING BEAM 3000 30F-E5 glulam beams use the same materials as 30F-E4 glulam beams except that outermost compression laminations are made of conventional 302-24 Douglas fir-Larch laminations. KING BEAM 3000 30F-E4 and 30F-E5 glulam beams are manufactured in widths of 3¹/₄, 5¹/₄, and 7 inches (83, 133, and 178 mm) with a minimum depth of 7¹/₄ inches (184 mm) and a maximum depth of 54 inches (1372 mm).

3.1.2 BOISE GLULAM: BOISE GLULAM are glulam beams and columns and are manufactured in accordance with ANSI A190.1 and based on layup combinations published in ANSI 117. BOISE GLULAM beams, except 24F-V4M3/DF, are manufactured in widths of 3¹/₈ to 20¹/₄ inches (79 to 514 mm) with a minimum depth of 6 inches (152 mm) and lengths up to 66 feet (20 116 mm). BOISE GLULAM beams 24F-V4M3/DF are manufactured in widths of 3¹/₈ to 20¹/₄ inches (79 to 514 mm), depths of 4¹/₂ inches

(114 mm), and lengths up to 66 feet (20 116 mm). BOISE GLULAM columns are manufactured in widths of 3¹/₈ to 10³/₄ inches (79 to 273 mm), depths of 5¹/₈ to 12 inches (130 to 305 mm), and lengths up to 30 feet (9144 mm)

3.2 Adhesive: Face and end-joint bonding adhesives comply with ASTM D2559 and ASTM D7247 for exterior or wet use.

3.3 End Joints: End joints comply with ANSI A190.1.

3.4 Laminating Lumber: KING BEAM 3000 and BOISE GLULAM beams and columns are manufactured using visually graded or E-rated lumber laminations that comply with rules of applicable approved lumber grading agencies and the procedures specified in the approved manufacturer's quality control manual in accordance with ANSI 117 and ANSI A190.1.

3.5 Layup Combinations:

3.5.1 KING BEAM 3000 is developed in accordance with ANSI 117 Alternate Combinations where KINGBEAM 3000 uses special manufactured Douglas fir-Larch lumber tension laminations using a proprietary process and strict quality controls.

3.5.2 BOISE GLULAM beams and columns are fabricated in accordance with ANSI A190.1 using the grade combinations noted in [Table 1](#) or single grade layups noted in [Table 2](#) of this report.

4.0 DESIGN AND INSTALLATION

The KING BEAM 3000 and BOISE GLULAM beam and columns described in this evaluation report comply with the building codes and are permitted to be designed in accordance with the National Design Specification for Wood Construction (NDS) or ANSI 117 using the allowable stress design (ASD) properties in [Tables 1](#) and [2](#) for beam and column application, respectively. Design values for the Load and Resistance Factor Design (LRFD) shall be permitted to be converted from the ASD values using the format conversion factors provided in Appendix N of the NDS.

The BOISE GLULAM shall be permitted to be installed flatwise and used as a diaphragm to resist lateral forces without wood structural panel sheathing. Such diaphragms shall be permitted to be designed in accordance with Section 4.5 Cross-Laminated Timber (CLT) Diaphragms of the ANSI/AWC Special Design Provisions for Wind and Seismic (SDPWS). Shear capacity for dowel type fasteners shall be designed utilizing appropriate specific gravities listed in [Table 2](#). Diaphragm deflection shall be determined using principles of engineering mechanics.

5.0 CONDITIONS OF USE:

The glulam combinations described in this report comply with or are suitable alternatives to what is specified in those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1** KING BEAM 3000 glulam beams shall have a minimum depth of 71/4 inches (181 mm) and a maximum depth of 54 inches (1372 mm). BOISE GLULAM beams and columns shall have a minimum depth of 6 inches (152 mm).
- 5.2** Fabrication, design, and installation must comply with this evaluation report and the manufacturer's published design/installation instructions, and the applicable code. In the event of a conflict between this evaluation report and the manufacturer's design/published installation instructions, the most restrictive one governs.
- 5.3** The manufacturer's design and/or installation instructions must be available at the jobsite at all times during installation.
- 5.4** ASD referenced design values are for normal loading conditions and must not exceed those provided in [Tables 1](#) and [2](#).
- 5.5** ASD referenced design values for combinations noted in [Table 1](#) are for members with four or more laminations stressed primarily in bending due to loads applied perpendicular to the wide faces of the laminations. The referenced design values are included, however, for axial stresses and stresses from bending due to loads applied parallel to the wide faces of the laminations.
- 5.6** ASD referenced design values for combinations noted in [Table 2](#) are for members with two or more laminations stressed primarily axially or in bending due to loads applied parallel to the wide faces of the laminations. The referenced design values are included, however, for stresses from bending due to loads applied perpendicular to the wide faces of the laminations.

- 5.7 The construction documents, including drawings and design details, prepared and reviewed by a registered design professional to verify the compliance with this report and applicable codes, must be submitted to the code official for approval, when requested.
- 5.8 The effects of checking on the glulam members are outside the scope of this report.
- 5.9 KING BEAM 3000 and BOISE GLULAM beams and columns are manufactured under a quality assurance program with inspections by ICC-ES and APA—The Engineered Wood Association (AA-649) or PFS Corporation, operating as PFS TECO (AI-3954).

6.0 EVIDENCE SUBMITTED

- 6.1 Data in accordance with ANSI 117, ANSI A190.1, and ANSI 405.
- 6.2 Data in accordance with ASTM D198, ASTM D3737, ASTM D4761, ASTM D7374, and ASTM E455.
- 6.3 Quality documentation.

7.0 IDENTIFICATION

- 7.1 The ICC-ES mark of conformity, electronic labeling, or the evaluation report number (ICC-ES ESR-6004) along with the name, registered trademark, or registered logo of the report holder must be included in the product label.
- 7.2 In addition, KING BEAM 3000 and BOISE GLULAM beams and columns are identified by a stamp bearing the manufacturing plant number ([Table 3](#)) or address, the glulam combination identification symbol, and the name of the third-party inspection agency (APA—The Engineered Wood Association or PFS TECO).
- 7.3 The report holder's contact information is the following:

BOISE CASCADE WOOD PRODUCTS, LLC
1111 W JEFFERSON ST., SUITE 300
BOISE, IDAHO 83702
(541) 826-0200
www.bc.com/ewp

TABLE 1—ASD REFERENCE DESIGN VALUES FOR KING BEAM 3000 AND BOISE GLULAM BEAMS FOR NOMAL DURATION OF LOAD^(1,2,3)

Symbol	Species Outer/ Core ⁽⁴⁾ (Bal or Unbal ⁽⁵⁾)	Bending About X-X Axis (Loaded Perpendicular to Wide Faces of Laminations)								Bending About Y-Y Axis (Loaded Parallel to Wide Faces of Laminations)						Axially Loaded		Fasteners	
		Extreme Fiber in Bending ⁽⁶⁾		Compression Perpendicular to Grain		Shear Parallel to Grain ⁽⁷⁾	Modulus of Elasticity ⁽⁸⁾			Extreme Fiber in Bending ⁽⁹⁾	Comp. Perpendicular to Grain	Shear Parallel to Grain ⁽⁷⁾	Modulus of Elasticity ⁽⁸⁾			Tension Parallel to Grain	Comp. Parallel to Grain	Specific Gravity for Dowel-Type Fastener Design	
		Bottom of Beam Stressed in Tension (Positive Bending)	Top of Beam Stressed in Tension (Negative Bending)	Ten. Face	Comp. Face		True	Apparent	Beam Stability				True	Apparent	Beam Stability			Top or Bottom Face	Side Face
		F_{bx}^* (psi)	F_{bx}^* (psi)	F_{cLx} (psi)		F_{vx} (psi)	$E_{x \text{ true}} (10^6 \text{ psi})$	$E_{x \text{ app}} (10^6 \text{ psi})$	$E_{x \text{ min}} (10^6 \text{ psi})$	F_{by} (psi)	F_{cLy} (psi)	F_{vy} (psi)	$E_{y \text{ true}} (10^6 \text{ psi})$	$E_{y \text{ app}} (10^6 \text{ psi})$	$E_{y \text{ min}} (10^6 \text{ psi})$	F_t (psi)	F_c (psi)	SG	
KING BEAM 3000 30F-E4 ⁽¹⁰⁾	DF/SP (B)	3,000	3,000	805	805	300	2.4	2.3	1.22	1,900	650	240	2.1	2.0	1.06	1,450	1,800	0.55	0.53 ⁽¹¹⁾
KING BEAM 3000 30F-E5 ⁽¹⁰⁾	DF/SP (U)	3,000	2,600	805	805	300	2.4	2.3	1.22	1,900	650	240	2.1	2.0	1.06	1,450	1,800	0.55	0.53 ⁽¹¹⁾
24F-V4	DF/DF (U)	2,400	1,850	650	650	265	1.9	1.8	0.95	1,450	560	230	1.7	1.6	0.85	1,100	1,650	0.50	0.50
24F-V4M3 ⁽¹²⁾	DF/DF (U)	2,400	1,850	650	650	265	1.9	1.8	0.95	1,450	560	230	1.7	1.6	0.85	1,100	1,650	0.50	0.50
24F-V8	DF/DF (B)	2,400	2,400	650	650	265	1.9	1.8	0.95	1,550	560	230	1.7	1.6	0.85	1,100	1,650	0.50	0.50
22F-V14	POC/POC (U)	2,200	1,650	560	560	265	1.8	1.7	0.90	1,700	470	230	1.7	1.6	0.85	1,200	2,100	0.46	0.46
22F-V15	POC/POC (B)	2,200	2,200	560	560	265	1.8	1.7	0.90	1,700	470	230	1.7	1.6	0.85	1,200	2,100	0.46	0.46
20F-V12	AC/AC (U)	2,000	1,400	560	560	265	1.6	1.5	0.79	1,250	470	230	1.5	1.4	0.74	925	1,500	0.46	0.46
20F-V13	AC/AC (B)	2,000	2,000	560	560	265	1.6	1.5	0.79	1,250	470	230	1.5	1.4	0.74	950	1,550	0.46	0.46
20F-V14	POC/POC (U)	2,000	1,450	560	560	265	1.6	1.5	0.79	1,300	470	230	1.5	1.4	0.74	900	1,600	0.46	0.46
20F-V15	POC/POC (B)	2,000	2,000	560	560	265	1.6	1.5	0.79	1,300	470	230	1.5	1.4	0.74	900	1,600	0.46	0.46
16F-V3	DF/DF (U)	1,600	1,250	560	560	265	1.6	1.5	0.79	1,450	560	230	1.6	1.5	0.79	975	1,500	0.50	0.50
16F-V6	DF/DF (B)	1,600	1,600	560	560	265	1.7	1.6	0.85	1,450	560	230	1.6	1.5	0.79	1000	1,600	0.50	0.50
Wet-use factor		0.8		0.53		0.875	0.833			0.8	0.53	0.875	0.833			0.8	0.73	see NDS	

For SI: 1 inch = 25.4 mm, 1 foot = 0.305 m, 1 psi = 6894.8 Pa

- (1) The combinations in this table, except 24F-V4M3/DF, are applicable to members consisting of 4 or more laminations and are intended primarily for members stressed in bending due to loads applied perpendicular to the wide faces of the laminations. Allowable design values are tabulated, however, for loading both perpendicular and parallel to the wide faces of the laminations.
- (2) The tabulated allowable design values are for normal duration of loading. For other durations of loading, see the applicable building code. The tabulated allowable design values are for dry conditions of use. For wet conditions of use, multiply the tabulated values by the wet-use factors shown at the bottom of the table.
- (3) Referenced design values must be adjusted, as applicable, in accordance with Section 5.3 of the NDS.
- (4) AC = Alaska cedar, DF = Douglas fir-Larch, POC = Port Orford cedar, and SP = Southern pine.
- (5) The unbalanced (U) layup is intended primarily for simple-span applications and the balanced (B) layup is intended primarily for continuous or cantilevered applications.
- (6) The values of F_{bx} are based on members 5 1/8 inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, F_{bx} shall be multiplied by a volume factor, $C_v = (5.125/b)^{1/10} (12/d)^{1/10} (21/L)^{1/10}$, where b is the beam width (in.), d is the beam depth (in.), and L is the beam length between the points of zero moment (ft).
- (7) For non-prismatic members, members subject to impact or cyclic loading, or shear design of bending members at connections (2024 NDS 3.4.4.1 or 2018 and 2015 NDS 3.4.3.3), the F_{vx} and F_{vy} values shall be multiplied by a factor of 0.72. The tabulated F_{vy} values are for timbers with laminations made from a single piece of lumber across the width or multiple pieces that have been edge bonded. For timber manufactured from multiple piece laminations (across width) that are not edge bonded, value shall be multiplied by 0.4 for members with 5, 7, or 9 laminations or by 0.5 for all other members.
- (8) The tabulated E values include true E (also known as "shear-free E "), apparent E , and E for beam stability calculation (NDS 3.3.3.8). For calculating beam deflections, the tabulated E_{app} values shall be used unless the shear deflection is determined in addition to bending deflection based on the tabulated E_{true} . The axial modulus of elasticity, E_{axial} and $E_{axial \text{ min}}$, shall be equal to the tabulated $E_{y \text{ true}}$ and $E_{y \text{ min}}$ values.
- (9) The values of F_{by} are based on members 12 inches in depth. For depths less than 12 inches, F_{by} shall be permitted to be increased by multiplying by the flat use factor, $(12/d)^{1/9}$, where d is the beam depth in inches. When d is less than 3 inches, use the size adjustment factor for 3 inches.
- (10) The beam depths are limited to 7 1/4 to 54 inches.
- (11) The specific gravity shall be permitted to be increased to 0.55 when the fastener is installed in the outer 25% (top and bottom) and the center 40% of the beam depth.
- (12) This combination only applies to members with 3 laminations.

TABLE 2—ALLOWABLE DESIGN VALUES FOR BOISE GLULAM COLUMNS FOR NORMAL DURATION OF LOAD^(1,2)

Combination Symbol	Species ⁽³⁾	Grade	All Loading				Axially Loaded			Bending about Y-Y Axis				Bending about X-X Axis		Fasteners
			Modulus of Elasticity ⁽⁴⁾			Compression Perpendicular to Grain	Tension Parallel to Grain	Compression Parallel to Grain		Loaded Parallel to Wide Faces of Laminations			Loaded Perpendicular to Wide Faces of Laminations		Specific Gravity for Dowel-Type Fastener Design	
							2 or More Lams	4 or More Lams	2 or 3 Lams	Bending ⁽⁵⁾			Shear Parallel to Grain ^(6,7)	Bending ⁽⁸⁾ 2 Lams to 15 in. Deep ⁽⁹⁾		Shear Parallel to Grain ⁽⁶⁾
			E _{x true} , E _{y true} or E _{axial} (10 ⁶ psi)	E _{x app} or E _{y app} (10 ⁶ psi)	E _{x min} , E _{y min} or E _{axial min} (10 ⁶ psi)	F _{cL} (psi)	F _t (psi)	F _c (psi)	F _c (psi)	F _{by} (psi)	F _{by} (psi)	F _{by} (psi)	F _{vy} (psi)	F _{bx} (psi)	F _{vx} (psi)	SG
1	DF	L3	1.6	1.5	0.79	560	950	1,550	1,250	1,450	1,250	1,000	230	1,250	265	0.50
2 ⁽¹⁰⁾	DF	L2	1.7	1.6	0.85	560	1,250	1,950	1,600	1,800	1,600	1,300	230	1,700	265	0.50
3	DF	L2D	2.0	1.9	1.00	650	1,450	2,300	1,900	2,100	1,850	1,550	230	2,000	265	0.50
5	DF	L1	2.1	2.0	1.06	650	1,650	2,400	2,100	2,400	2,100	1,800	230	2,200	265	0.50
70	AC	L2	1.4	1.3	0.69	470	975	1,450	1,450	1,400	1,250	1,000	230	1,350	265	0.46
74	POC	L2	1.5	1.4	0.74	470	1,050	1,900	1,550	1,450	1,300	1,100	230	1,400	265	0.46
Wet-use factors			0.833			0.53	0.8	0.73		0.8			0.875	0.8	0.875	see NDS

For **SI**: 1 inch = 25.4 mm, 1 foot = 0.305 m, 1 psi = 6894.8 Pa

- (1) The tabulated allowable design values are for normal duration of loading. For other durations of loading, see applicable building code. The tabulated allowable design values are for dry conditions of use. For wet conditions of use, multiply the tabulated values by the factors shown at the bottom of the table.
- (2) Referenced design values must be adjusted, as applicable, in accordance with Section 5.3 of the NDS.
- (3) AC = Alaska cedar, DF = Douglas fir-Larch, and POC = Port Orford cedar.
- (4) The tabulated E values include shear-free (true) modulus of elasticity ($E_{x \text{ true}}$, $E_{y \text{ true}}$, and E_{axial}), apparent modulus of elasticity ($E_{x \text{ app}}$ and $E_{y \text{ app}}$), and 5th percentile modulus of elasticity ($E_{x \text{ min}}$, $E_{y \text{ min}}$, and $E_{\text{axial min}}$). For column stability calculation (NDS 3.7.1), $E_{\text{axial min}}$ shall be used. For calculating the total deflection due to bending, the tabulated $E_{x \text{ app}}$ or $E_{y \text{ app}}$ values shall be used, or as an alternative, the true (shear-free) bending deflection shall be calculated using the tabulated $E_{x \text{ true}}$ or $E_{y \text{ true}}$, which shall be added to the calculated shear deflection to determine the total deflection due to bending.
- (5) The values of F_{by} are based on members 12 inches in depth. For depths less than 12 inches, F_{by} shall be permitted to be increased by multiplying by the flat use factor, $(12/d)^{1/9}$, where d is the beam depth in inches. When d is less than 3 inches, use the size adjustment factor for 3 inches.
- (6) For non-prismatic members, notched members, members subject to impact or cyclic loading, or shear design of bending members at connections (2024 NDS 3.4.4.1 or 2018 and 2015 NDS 3.4.3.3), the tabulated F_{vx} and F_{vy} values shall be multiplied by 0.72.
- (7) The tabulated F_{vy} values are for members of 4 or more lams. The tabulated F_{vy} values shall be multiplied by a factor of 0.95 for 3 lams and 0.84 for 2 lams. For members with 5, 7, or 9 lams manufactured from multiple-piece lams with unbonded edge joints, the tabulated F_{vy} values shall be multiplied by a factor of 0.4. For all other members manufactured from multiple-piece lams with unbonded edge joints, the tabulated F_{vy} values shall be multiplied by a factor of 0.5. This adjustment shall be cumulative with the adjustment specified in Footnote 6.
- (8) The values of F_{bx} are based on members 5¹/₈ inches in width by 12 inches in depth by 21 feet in length. For members with a larger volume, F_{bx} shall be multiplied by a volume factor, $C_v = (5.125/b)^{1/10} (12/d)^{1/10} (21/L)^{1/10}$, where b is the beam width (in.), d is the beam depth (in.), and L is the beam length between the points of zero moment (ft).
- (9) The tabulated F_{bx} values are for members without special tension lams up to 15 inches in depth. If the member depth is greater than 15 inches without special tension lams, the tabulated F_{bx} values must be multiplied by a factor of 0.88. If special tension lams are used, the tabulated F_{bx} values are permitted to be increased by a factor of 1.18 regardless of the member depth provided that the increased F_{bx} value does not exceed 2,400 psi. This factor shall be cumulative with the volume factor, C_v , specified in Footnote 8.
- (10) Vertically curved (horizontally laminated) beams utilizing all 302 Douglas fir tension laminations and with individual laminations planed down to a thickness, where $5/8$ inch $\leq$ thickness $< 13/8$ inches, shall be permitted to be trademarked and designed as a Combination #2 glulam.

TABLE 3 – MANUFACTURING LOCATIONS

APA Mill Number	Plant Address
1107	4318 Pioneer Rd, Homedale, Idaho 83628



FIGURE 1—COMPANY LOGO

ICC-ES Evaluation Report

ESR-6004 City of LA Supplement

Issued September 2026

Revised October 2026

This report remains under an active [continuous compliance program](#) through the end of September 2027.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES

Section: 06 02 00—Design Information

Section: 06 18 13—Glued-Laminated Beams

REPORT HOLDER:

BOISE CASCADE WOOD PRODUCTS, LLC

EVALUATION SUBJECT:

KING BEAM 3000 AND BOISE GLULAM

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that KING BEAM 3000 and BOISE GLULAM beams and columns, described in ICC-ES evaluation report [ESR-6004](#), have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2026 City of Los Angeles Building Code ([LABC](#))
- 2026 City of Los Angeles Residential Code ([LARC](#))

2.0 CONCLUSIONS

KING BEAM 3000 and BOISE GLULAM beams and columns, described in Sections 2.0 through 7.0 of the evaluation report [ESR-6004](#), comply with the LABC Chapter 23, and the LARC, and are subject to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

KING BEAM 3000 and BOISE GLULAM beams and columns described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report [ESR-6004](#).
- The design, installation, conditions of use and identification of the Vaagen structural glued-laminated timber products are in accordance with the 2024 *International Building Code*® (IBC) and 2024 *International Residential Code*® (IRC) provisions, as applicable, noted in the evaluation report [ESR-6004](#).
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16, 17 and 23, as applicable.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.

This evaluation report supplement expires concurrently with the evaluation report [ESR-6004](#), issued September 2026 and revised October 2026.

ICC-ES Evaluation Report

ESR-6004 CA Supplement

Issued September 2026

Revised October 2026

This report remains under an active [continuous compliance program](#) through the end of September 2027.

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES

Section: 06 02 00—Design Information

Section: 06 18 13—Glued-Laminated Beams

REPORT HOLDER:

BOISE CASCADE WOOD PRODUCTS, LLC

EVALUATION SUBJECT:

KING BEAM 3000 AND BOISE GLULAM

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that KING BEAM 3000 and BOISE GLULAM beams and columns, described in the ICC-ES evaluation report [ESR-6004](#), have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2025 *California Building Code* (CBC)

For evaluation of applicable chapters adopted by the California Office of Statewide Health Planning and Development (OSHPD) AKA: California Department of Health Care Access and Information (HCAI) and the Division of State Architect (DSA), see Sections 2.1.1 and 2.1.2 below.

- 2025 *California Residential Code* (CRC)

2.0 CONCLUSIONS

2.1 CBC:

KING BEAM 3000 and BOISE GLULAM beams and columns, described in Sections 2.0 through 7.0 of the ICC-ES evaluation report [ESR-6004](#), comply with CBC Chapter 23, provided the design and installation are in accordance with the 2024 *International Building Code*® (IBC) provisions noted in the evaluation report and the additional requirements of CBC Chapters 16, 17 and 23, as applicable.

2.1.1 OSHPD:

The applicable OSHPD Sections and Chapters of the CBC are beyond the scope of this supplement.

2.1.2 DSA:

The applicable DSA Sections Chapters of the CBC are beyond the scope of this supplement.

2.2 CRC:

KING BEAM 3000 and BOISE GLULAM beams and columns, described in Sections 2.0 through 7.0 of the ICC-ES evaluation report [ESR-6004](#), comply with the CRC, provided the design and installation are in accordance with the 2024 *International Residential Code*® (IRC) provisions noted in the evaluation report and the additional requirements of CRC Chapter 3, as applicable.

This supplement expires concurrently with the evaluation report [ESR-6004](#), issued September 2026 and revised October 2026.